

**GoldRich** PLANNERS & SURVEYORS LTD.

金潤規劃測量師行有限公司

Your Ref.: A/YL-KTS/1090

Our Ref.: P25040/TL25323

30 September 2025

The Secretary  
Town Planning Board  
15/F., North Point Government Offices  
333 Java Road, North Point, Hong Kong

By Post and E-mail  
tpbpd@pland.gov.hk

Dear Sir,

**Submission of Further Information (FI)**

**Proposed Temporary Private Vehicle Park (Private Cars Only)  
for a Period of 3 Years in “Village Type Development” Zone,  
Lot 343 (Part) in D.D. 113, Kam Tin, Yuen Long, New Territories  
(Application No. A/YL-KTS/1090)**

We write to submit FI in response to departmental comment(s) conveyed by the Planning Department for the captioned application.

Yours faithfully,  
For and on behalf of  
Goldrich Planners & Surveyors Ltd.



Francis LAU

Encl.

c.c.  
DPO/FS&YLE, PlanD (Attn.: Ms. Anna TONG) *By E-mail only*

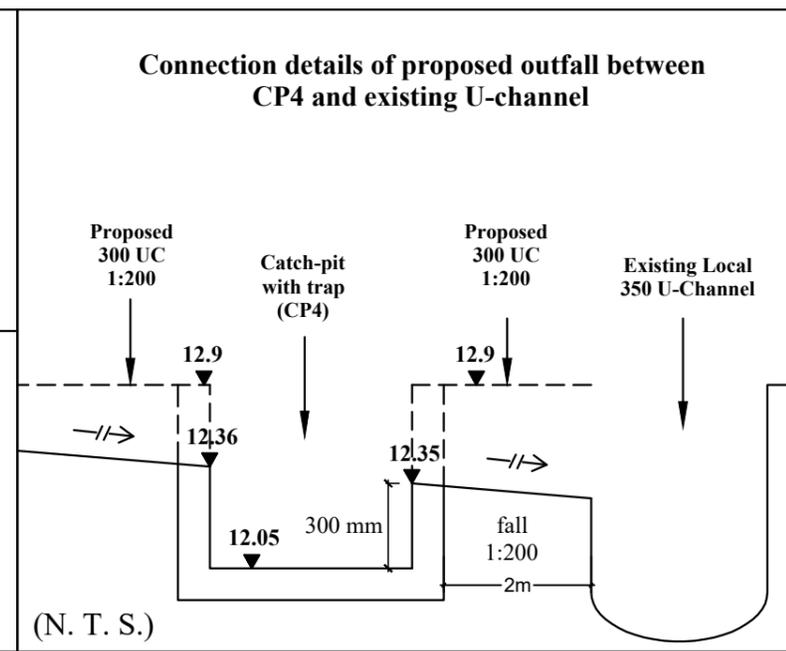
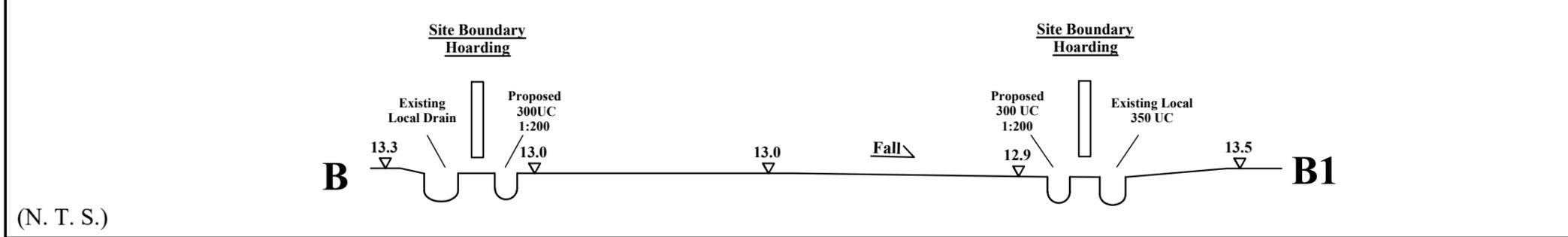
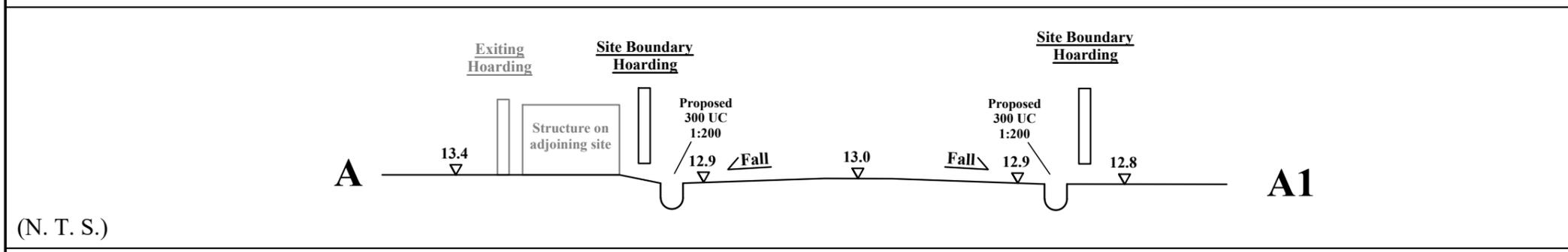
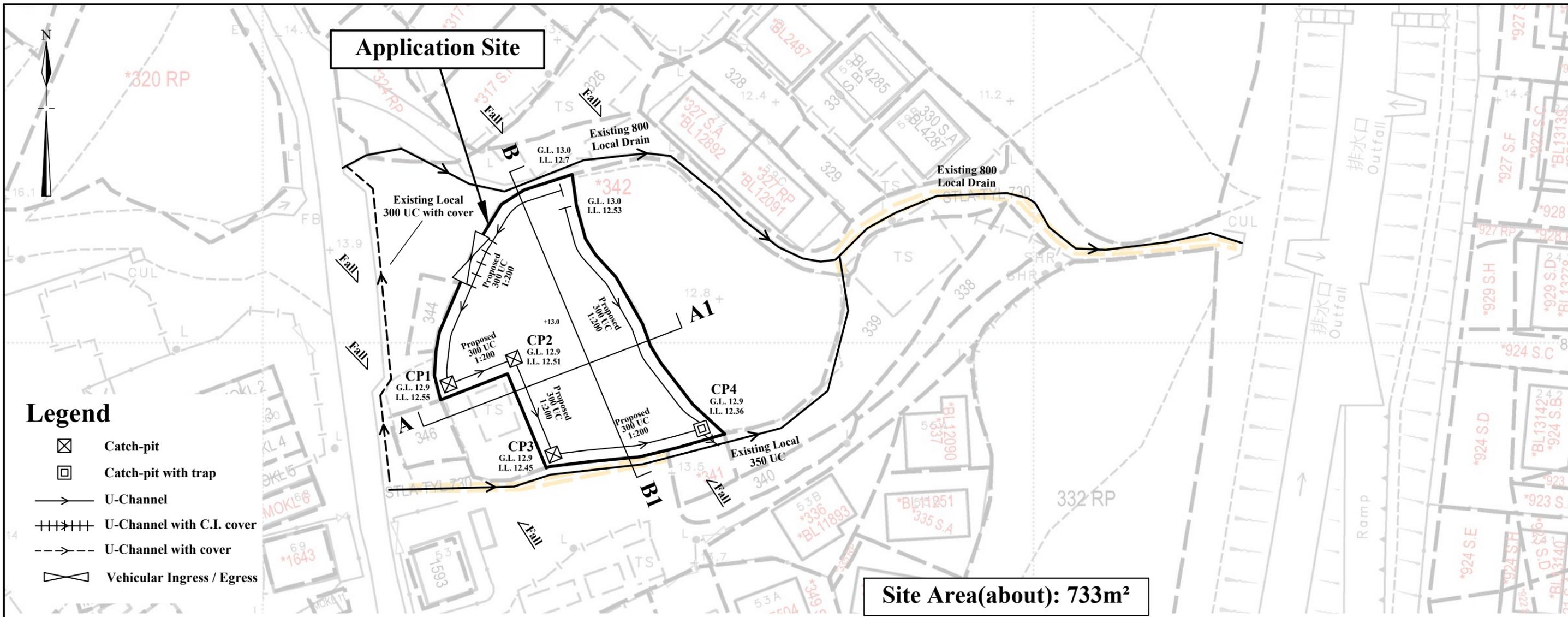
**Further Information for Planning Application No. A/YL-KTS/1090**  
**Response-to-Comments**

**Comments from Chief Engineer/Mainland North, Drainage Services Department**

Contact person: Mr. CHAN Yue Lap, Kenneth (Tel.: 2300 1259)

<b>I.</b>	<b>Comments</b>	<b>Responses</b>
1.	According to his record, there are existing streamcourse/channel on government land within the application site that may also serve the adjacent land. There is a potential increase flooding risk to the area if the government land and the streamcourse/channel are disturbed or affected by the operation of the applicant. The applicant shall submit a drainage proposal as mentioned in paragraph 19 of the planning statement for his further review.	Please see the drainage proposal ( <b>Plans 5.1 &amp; 5.2</b> ) and hydraulic calculation for details.

- END -



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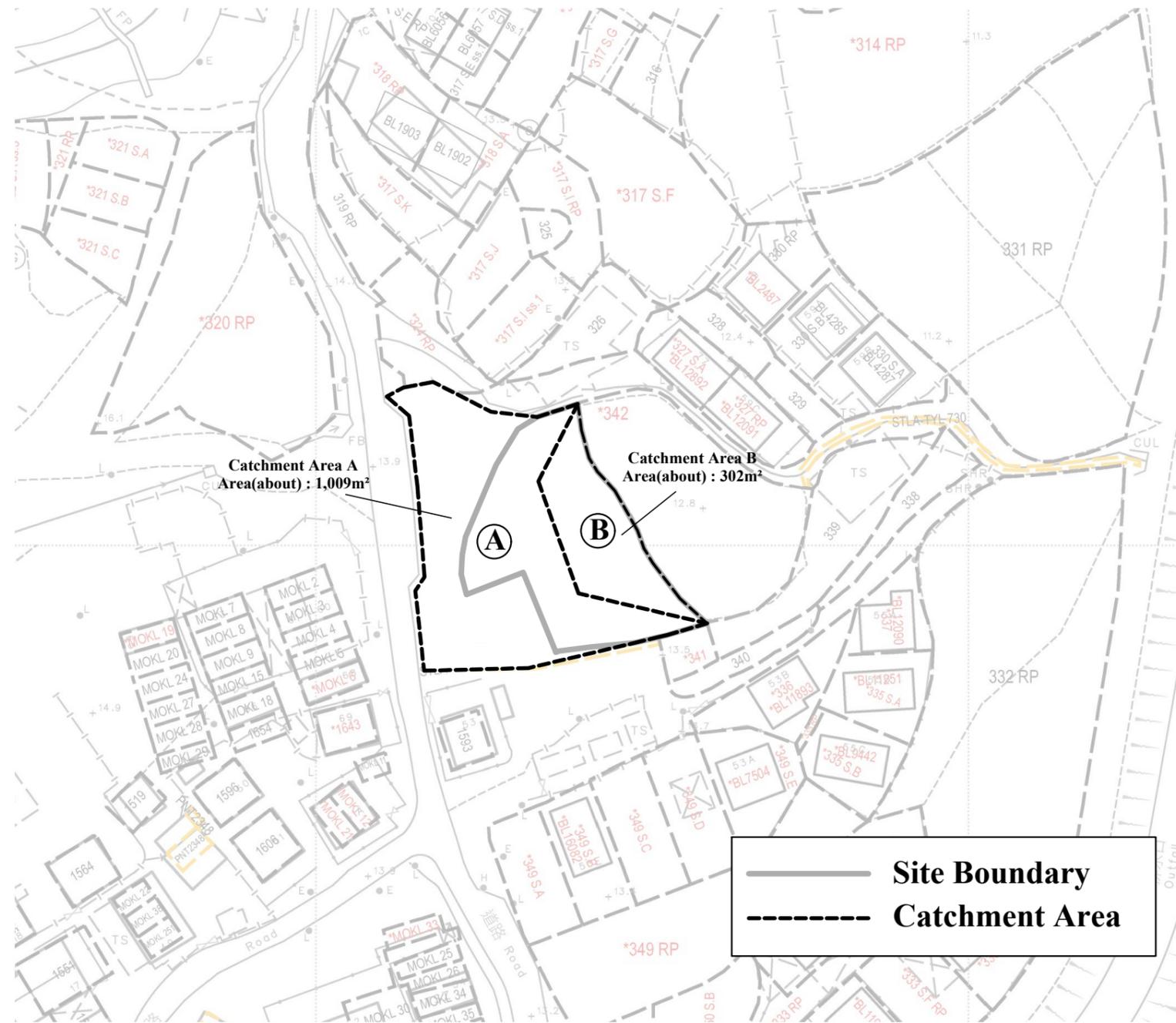
September 2025

# Drainage Proposal

Lot 343(Part) in D.D. 113

**Goldrich Planners & Surveyors Ltd.**

**Plan 5.1**  
( P 25040 )

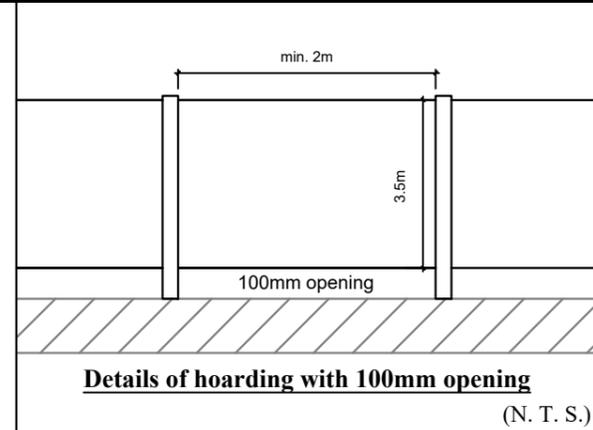


Catchment Area A  
Area (about) : 1,009m<sup>2</sup>

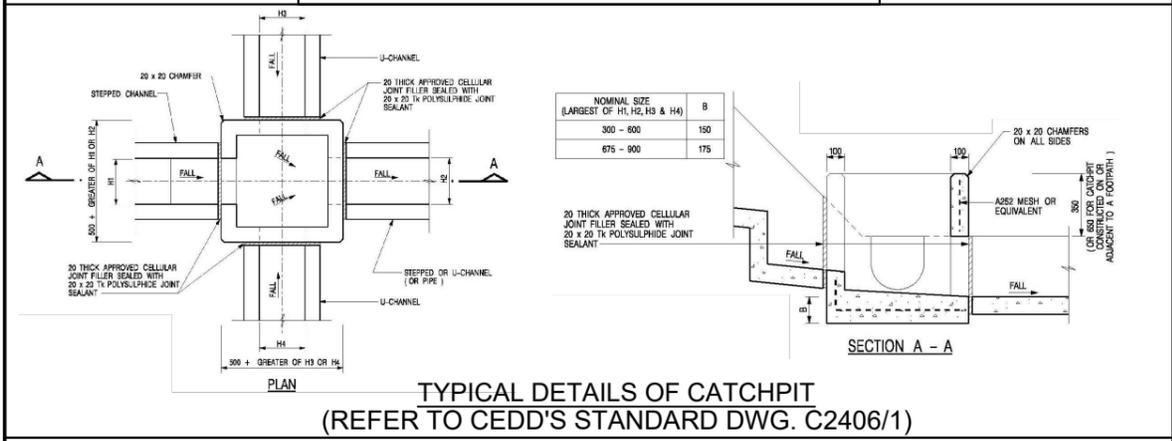
Catchment Area B  
Area (about) : 302m<sup>2</sup>

**AREA OF CATCHMENT**  
(N.T.S)

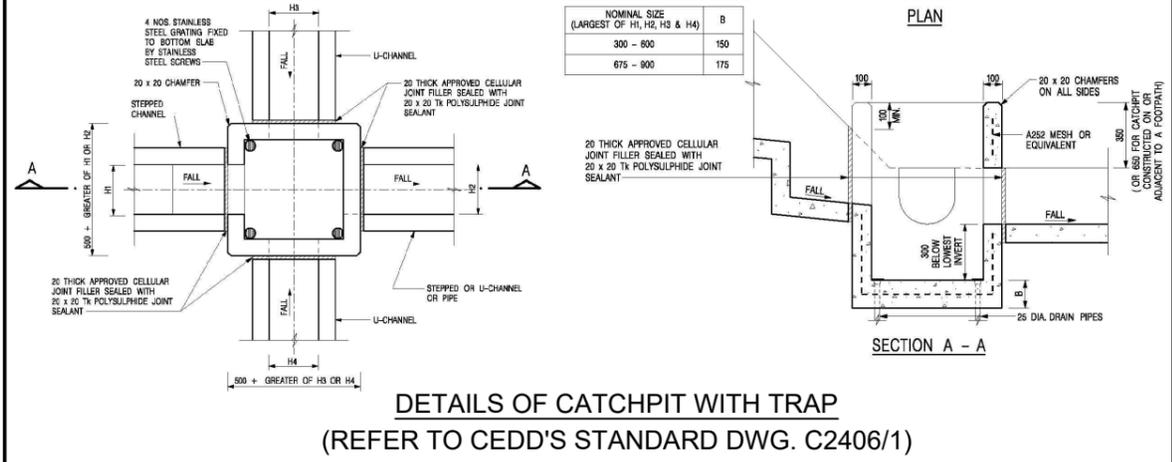
— Site Boundary  
- - - Catchment Area



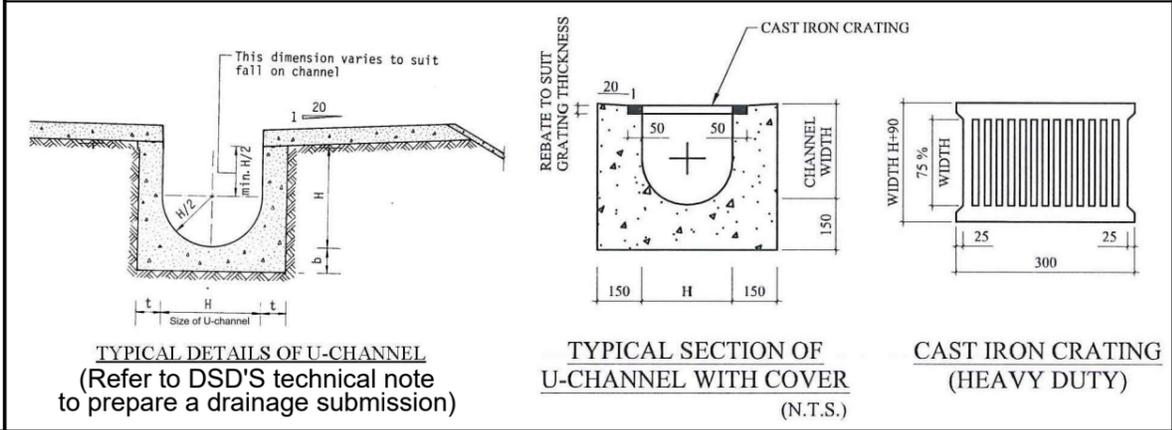
**Details of hoarding with 100mm opening**  
(N. T. S.)



**TYPICAL DETAILS OF CATCHPIT**  
(REFER TO CEDD'S STANDARD DWG. C2406/1)



**DETAILS OF CATCHPIT WITH TRAP**  
(REFER TO CEDD'S STANDARD DWG. C2406/1)



**TYPICAL DETAILS OF U-CHANNEL**  
(Refer to DSD'S technical note to prepare a drainage submission)

**TYPICAL SECTION OF U-CHANNEL WITH COVER**  
(N.T.S.)

**CAST IRON CRATING (HEAVY DUTY)**

N.T.S

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**Drainage Proposal**  
**Lot 343(Part) in D.D. 113**

**Goldrich Planners & Surveyors Ltd.**

**Plan 5.2**  
**( P 25040 )**

1 For Catchment Area A

Area, A = 1009 m<sup>2</sup>  
 Average slope, H = 0.1 m per 100m  
 Distance on the line of natural flow, L = 18 m

Time of concentration,  $t_o = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (18) / (0.1^{0.2} \times 1009^{0.1})$   
 = 2.1 min

SDM 7.5.2 (d)

2 For Proposed UC in Catchment Area A

	From	To
Ground level (mPD)	13.00	12.90
Invert level (mPD)	12.70	12.36

Width of u-channel, w = 300 mm  
 Length of u-channel, L<sub>c</sub> = 67.6 m  
 Depth of vertical part of u-channel, d = 390 mm  
 Gradient of u-channel, S<sub>f</sub> = (12.7-12.36)/67.6 = 0.005

Cross-Section Area, a =  $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 150^2 + 300 \times 390$   
 = 0.152 m<sup>2</sup>  
 Wetted Perimeter, p =  $\pi r + 2 d = 3.14 \times 150 + 2 \times 390$   
 = 1.251 m  
 Hydraulic radius, R = a / p  
 = 0.122 m

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-  
 Allowable velocity, v =  $R^{1/6} \times (RS_f)^{1/2} / n = (0.122)^{1/6} \times (0.122 \times 0.005)^{1/2} / 0.016$   
 = 1.09 m/s  
 Time of flow, t<sub>f</sub> = 1.0 min

SDM Table 13  
 SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i =  $a / (t_o + t_f + b)^c$   
 =  $505.5 / (2.1 + 1 + 3.29)^{0.355}$  for return period T = 50 years  
 = 262

SDM 4.3.2  
 Corrigendum 1/2024  
 SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m <sup>2</sup> )	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	1009.0	958.6
SUM =			958.6

SDM 7.5.2 (b)

Upstream flow, Q<sub>u</sub> = 0 m<sup>3</sup>/s

Design flow, Q<sub>d</sub> =  $0.278i \sum C_i A_i + Q_u$  where A<sub>i</sub> is in km<sup>2</sup>  
 =  $0.278 \times 262 \times 958.55 / 1000000 + 0$   
 = 0.070 m<sup>3</sup>/s

SDM 7.5.2 (a)

Allowable flow, Q<sub>a</sub> = a x v  
 = 0.152 x 1.09  
 = 0.166 m<sup>3</sup>/s

> Q<sub>d</sub> (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

Scale: NA

**Hydraulic Calculation**

Goldrich Planners &  
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September 2025

Lot 343 (Part) in D.D. 113, Kam Tin, Yuen Long, New Territories

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1 For Catchment Area B

Area, A = 302 m<sup>2</sup>  
 Average slope, H = 0.1 m per 100m  
 Distance on the line of natural flow, L = 10 m

Time of concentration,  $t_0 = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (10) / (0.1^{0.2} \times 302^{0.1})$   
 = 1.3 min

SDM 7.5.2 (d)

2 For Proposed UC in Catchment Area B

	From	To
Ground level (mPD)	13.00	12.90
Invert level (mPD)	12.53	12.36

Width of u-channel, w = 300 mm  
 Length of u-channel, L<sub>c</sub> = 33.4 m  
 Depth of vertical part of u-channel, d = 390 mm  
 Gradient of u-channel, S<sub>f</sub> = (12.53-12.36)/33.4 = 0.005

Cross-Section Area, a =  $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 150^2 + 300 \times 390$   
 = 0.152 m<sup>2</sup>  
 Wetted Perimeter, p =  $\pi r + 2 d = 3.14 \times 150 + 2 \times 390$   
 = 1.251 m  
 Hydraulic radius, R = a / p  
 = 0.122 m

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-  
 Allowable velocity, v =  $R^{1/6} \times (RS_f)^{1/2} / n = (0.122)^{1/6} \times (0.122 \times 0.005)^{1/2} / 0.016$   
 = 1.10 m/s  
 Time of flow, t<sub>f</sub> = 0.5 min

SDM Table 13  
 SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i =  $a / (t_0 + t_f + b)^c$   
 =  $505.5 / (1.3+0.5+3.29)^{0.355}$  for return period T = 50 years  
 = 284

SDM 4.3.2  
 Corrigendum 1/2024  
 SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m <sup>2</sup> )	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	302.0	286.9
			SUM = 286.9

SDM 7.5.2 (b)

Upstream flow, Q<sub>u</sub> = 0 m<sup>3</sup>/s

Design flow, Q<sub>d</sub> =  $0.278i \sum C_i A_i + Q_u$  where A<sub>i</sub> is in km<sup>2</sup>  
 =  $0.278 \times 284 \times 286.9 / 1000000 + 0$   
 = 0.023 m<sup>3</sup>/s

SDM 7.5.2 (a)

Allowable flow, Q<sub>a</sub> = a x v  
 = 0.152 x 1.1  
 = 0.167 m<sup>3</sup>/s

> Q<sub>d</sub> (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

Scale: NA

### Hydraulic Calculation

Goldrich Planners &  
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September 2025

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1 For Connection between CP4 and Existing Local 350 UC

Area, A = 0 m<sup>2</sup>  
 Average slope, H = 0.1 m per 100m  
 Distance on the line of natural flow, L = 0 m

Time of concentration,  $t_o = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (0) / (0.1^{0.2} \times 0^{0.1})$   
 = 0.0 min

SDM 7.5.2 (d)

2 For Proposed UC in Connection between CP4 and Existing Local 350 UC

	From	To
Ground level (mPD)	12.90	12.90
Invert level (mPD)	12.36	12.35

Width of u-channel, w = 300 mm  
 Length of u-channel, L<sub>c</sub> = 2 m  
 Depth of vertical part of u-channel, d = 400 mm  
 Gradient of u-channel, S<sub>f</sub> = (12.36-12.35)/2 = 0.005

Cross-Section Area, a =  $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 150^2 + 300 \times 400$   
 = 0.155 m<sup>2</sup>  
 Wetted Perimeter, p =  $\pi r + 2 d = 3.14 \times 150 + 2 \times 400$   
 = 1.271 m  
 Hydraulic radius, R = a / p  
 = 0.122 m

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-  
 Allowable velocity, v =  $R^{1/6} \times (RS_f)^{1/2} / n = (0.122)^{1/6} \times (0.122 \times 0.005)^{1/2} / 0.016$   
 = 1.09 m/s  
 Time of flow, t<sub>f</sub> = 0.0 min

SDM Table 13  
 SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i =  $a / (t_o + t_f + b)^c$   
 =  $505.5 / (0+0+3.29)^{0.355}$  for return period T = 50 years  
 = 330

SDM 4.3.2  
 Corrigendum 1/2024  
 SDM Table 3a

Type of surface	Runoff Coefficient C	Catchment Area A (m <sup>2</sup> )	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	0.0	0.0
SUM =			0.0

SDM 7.5.2 (b)

Upstream flow, Q<sub>u</sub> = 0.093 m<sup>3</sup>/s

Design flow, Q<sub>d</sub> =  $0.278i \sum C_i A_i + Q_u$  where A<sub>i</sub> is in km<sup>2</sup>  
 =  $0.278 \times 330 \times 0 / 1000000 + 0.093$   
 = 0.093 m<sup>3</sup>/s

SDM 7.5.2 (a)

Allowable flow, Q<sub>a</sub> = a x v  
 = 0.155 x 1.09  
 = 0.169 m<sup>3</sup>/s

> Q<sub>d</sub> (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

Scale: NA

### Hydraulic Calculation

Goldrich Planners &  
 Surveyors Ltd.

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